

# EuroBeam from Greentram Software

## Typical calculations produced by the pre-release version

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EuroBeam 1.00x0

Draycott.eub

### Beam: Draycott example 2.2/2.7: Timber beam

Span: 4.25 m.

	Load name	Loading w1	Start x1	Loading w2	End x2	R1comp	R2comp
U G	Dead load	2.0/L	0		L	1.00	1.00
U QB	Imposed load	3.5/L	0		L	1.75	1.75
Total load (unfactored): 5.500 kN						<b>2.75</b>	<b>2.75</b>
(6.10): 7.950 kN						3.97	3.97

Load types: U:UDL; Load positions are measured in m. from R1  
Load durations: G:Dead; Qx: Imposed; QB: Office

Maximum B.M. = 4.22 kNm (6.10) at 2.13 m. from R1

Maximum S.F. = 3.97 kN (6.10) at R1

Mid-span deflections: Dead:  $2.00 \times 10^8/EI$  ( $E$  in  $N/mm^2$ ,  $I$  in  $cm^4$ )

Live:  $3.50 \times 10^8/EI$

Total:  $5.50 \times 10^8/EI$

Timber beam calculation to BS EN1995-1-1 using C16 timber

**Use 72 x 220 C16** 5.9 kg/m approx

$W_{el,v} = 580.8 \text{ cm}^3$   $I_v = 6,389 \text{ cm}^4$

Depth factor,  $k_h = 1.00$  [EC5 3.2]

**Timber grade: C16**

Grade bending stress,  $f_{m,k} = 16.0 \text{ N/mm}^2$

Grade shear stress,  $f_{v,k} = 1.8 \text{ N/mm}^2$

Material partial factor,  $\gamma_M = 1.3$  [EC5 UK Table NA.3]

$E_{0,mean} = 8,000 \text{ N/mm}^2$

Loading modification factor,  $k_{mod} = 0.8$  (Service class 2; Live load duration: Medium term) [EC5 Table 3.1]

Load sharing factor,  $k_{sys} = 1.0$

Deflection modification factor,  $k_{def} = 0.800$  [EC5 Table 3.2]

#### Bending

Design bending stress,  $f_{m,d} = f_{m,k} \cdot k_{mod} \cdot k_h \cdot k_{sys} / \gamma_M = 16.0 \times 0.80 \times 1.00 \times 1.0 / 1.30 = 9.85 \text{ N/mm}^2$

Applied bending stress,  $\sigma_{m,y,d} = 4.22 \times 1000 / 580.8 = 7.27 \text{ N/mm}^2$  OK

#### Shear

Design shear stress,  $f_{v,d} = f_{v,k} \cdot k_{mod} \cdot k_{sys} / \gamma_M = 1.80 \times 0.80 \times 1.0 / 1.30 = 1.11 \text{ N/mm}^2$

Applied shear stress,  $\sigma_{v,y,d} = 3.97 \times 1000 \times 3/2 \times 72 \times 220 = 0.38 \text{ N/mm}^2$  OK

#### Deflection

Final deflection limit =  $L/150 = 28.33 \text{ mm}$

Instantaneous mid-span shear deflection =  $1.2 \times 2.92 \times 10^6 / ((E/16) \times 72 \times 220) = 0.44 \text{ mm}$

Final shear deflection is assumed to increase in proportion to total bending deflection

Mid-span deflections:	$\times 1e8/EI$	Inst. mm	$\psi_2$	kdef	Fin. mm
Dead:	2.00	3.91		0.80	7.04
Live QB:	3.50	6.84	0.30	0.80	8.49
Shear deflection:		0.44			0.64
Total		11.20			<b>16.17</b> OK

Check: Final deflection should be 16.2mm